

Data processing of displacements between the ground and the point of cracking at the seven – story Van Nuys Hotel

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Abstract - One of the most complicated and powerful systems at planet earth is the Mother Nature. Almost every day, somewhere in the world, an earthquake is registered with magnitudes that affects life of many humans. It is very important to better understand the system of the ground shaking caused by seismic waves. Collecting the data from these scenarios are very important for the researchers. We are processing the data from seven – story Hotel in Van Nuys, California during the Northridge Earthquake in 1994. During the earthquake, five instruments are measuring the displacements from the bottom to the top of the building. This paper represents the connection between the ground and the fifth floor at which the building is cracking. We will calculate some parameters that are important according to us between these two points and present graphically over the software LabView. Numerical solutions implemented via object – oriented blocks will give us the results that we are needed.

I. INTRODUCTION

One way to look at a signal is in the discrete time domain, which puts a series of values consecutively in time. In this way we can tell something about the behavior of the signal at every moment in time, and can also make some simple statements about its long-term behavior. However, it is rather difficult to say anything about how the long-term behavior is related to the short-term development of the signal. Another way to look at a signal is to view its spectral density (i.e., the Fourier transform of the signal). The Fourier transform views the signal as a whole. It swaps the dimension of time with the dimension of frequency. One can think of the Fourier transform as a combination of slow and fast oscillations with different amplitude. A very strong and slow component in the frequency domain implies that there is a high correlation between the large-scale pieces of the signal in time (macro-structures), while a very strong and fast oscillation implies correlation in the micro-structures. Therefore, if our signal represents values in every single moment of time, its Fourier transform represents the strength of every oscillation in a holistic way in that chunk of time.

A. The Building

This seven – story hotel in Van Nuys (VN7SH), California is one of the most studied buildings in southern California [2]. It has been designed in 1965 and constructed in 1966. VN7SH is located in central San Fernando Valley of the Los Angeles metropolitan area.

The building is 18.9×45.7 m in plan. The typical framing consists of columns spaced on 6.1 m centers in the transverse direction and 5.8 m centers in the longitudinal direction. Spandrel beams surround the perimeter of the structure (Figure 1). Lateral forces in the longitudinal (EW) direction are resisted by interior column-slab frames and exterior column spandrel beam frames [4]. The added stiffness in the exterior frames associated with the spandrel beams creates exterior frames that are roughly twice as stiff as interior frames. The floor system is reinforced concrete flat slab, 25.4 cm thick at the second floor, 21.6 cm thick at the third to seventh floors, and 20.3 cm thick at the roof.

The building is situated on undifferentiated Holocene alluvium, uncemented and unconsolidated, with a thickness of < 30 m, and an age of $< 10,000$ years [4]. The average shear-wave velocity in the top 30 m of soil is 300 m/s, and the soil-boring log shows that the underlying soil consists primarily of fine sandy silts and silty fine sands.

The foundation system consists of 96.5-cm deep pile caps, supported by groups of two to four poured-in-place 61-cm-diameter reinforced concrete friction piles. These are centered under the main building columns. All of the pile caps are connected by a grid of beams. Each pile is roughly 12.2 m long and has a design capacity of over 444.82×103 N vertical load and up to 88.96×103 N lateral load.



Fig. 1. View of Van Nuys Seven Story Hotel (VN7SH) from North-East.

TABLE 1. PROPERTIES OF THE CONSTRUCTION MATERIALS OF THE VN7SH BUILDING;

- (1) Pounds per cubic foot
(2) Pounds per square inch
(3) Kips per square inch

Concrete (regular weight, 150 pcf ⁽¹⁾)			
Location in the structure	Minimum specified compressive strength f'_c – psi ⁽²⁾	Modulus of elasticity E – psi ⁽²⁾	
Columns, 1 st to 2 nd floors	5,000	4.2×10^6	
Columns, 2 nd to 3 rd floors	4,000	3.7×10^6	
Beams and slabs, 2 nd floor	4,000	3.7×10^6	
All other concrete, 3 rd floor to roof	3,000	3.3×10^6	

Reinforcing steel			
Location in the structure	Grade	Minimum specified yield strength f_y – ksi ⁽³⁾	Modulus of elasticity E – psi ⁽²⁾
Beams and slabs	Intermediate grade deformed billet bars (ASTM A-15 and A-305)	40	29×10^6
Column bars	Deformed billet bars (ASTM A-432)	60	29×10^6

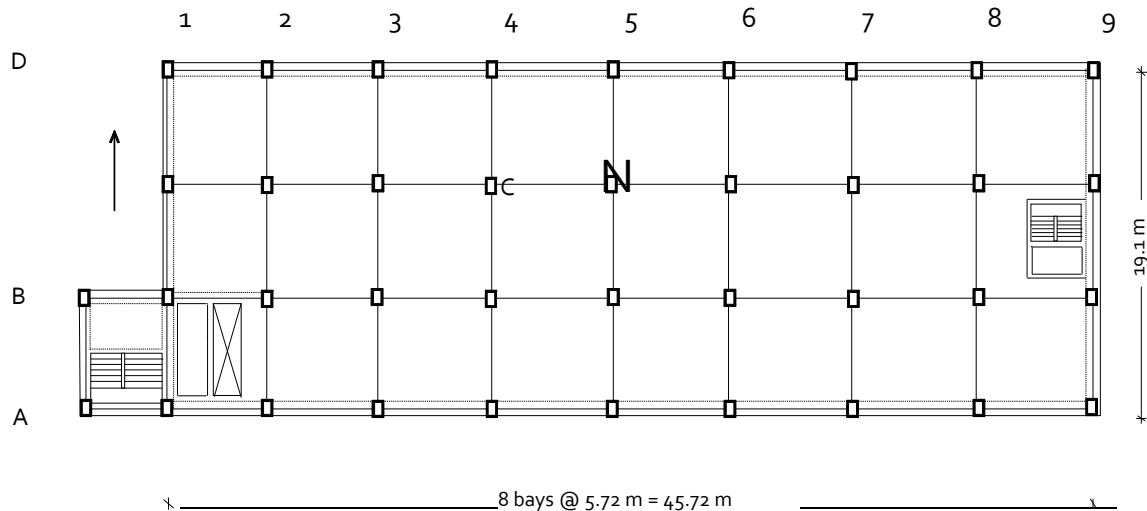


Fig. 2. Typical Floor Plan.

B. The Earthquake Damage

The $M_L = 6.4$ Northridge earthquake of January 17, 1994 severely damaged the building. The structural damage was extensive in the exterior north (D) and south (A) frames (figure 1) that were designed to take most of the lateral load in the longitudinal (EW) direction.

Severe shear cracks occurred at the middle columns of frame A, near the contact with the spandrel beam of the 5th floor (Figs. 1 and 2). Those cracks significantly decreased the axial, moment, and shear capacity of the columns. The shear cracks that appeared in the north (D) frame on the 3rd and 4th floors and the damage to columns D2, D3, and D4 on the 1st floor caused minor to moderate changes in the capacities of these structural elements. No major damage to the interior longitudinal (B and C) frames was observed, and there was no visible damage to the slabs or around the foundation. The nonstructural damage was also significant. The recorded peak accelerations in the building were 0.46g (L), 0.40g (T), and 0.28g (V) at the base, and 0.59g (L) and 0.58g (T) at the roof, along the longitudinal (L), transverse (T), and vertical (V) axes of symmetry (there were no sensors installed on the roof to measure vertical motions) [5].

During Northridge earthquake five transducers have measured the longitudinal displacements over the seven floors.

The response of VN7SH was recorded by a 13-channel CR-1 central recording system and by one tri-component SMA-1 accelerograph, with an independent recording system but with common trigger time with the CR-1 recorder.

The simplicity, uniformity, and symmetry of the building geometry make this building ideal for testing and for calibration of different analysis methods. Instead of the common earthquake several damages, the Van Nuys damage is concentrated at the fourth floor which makes VN7SH very important for any kind of numerical analyse.

We use digital signal processing via Labview to get known with the crucial characteristics of a seismic excitation like one in California 1994. In a study of the propagation of non-linear waves in a simple, uniform shear beam, caused by incident strong motion pulses, Gicev and Trifunac [2] found that for large ground displacement pulses the maximum permanent strains in the beam occur mainly at the interface of the beam with the soil, while for smaller amplitudes of pulses permanent strains occur closer to the top of the beam. They identified three zones of the permanently deformed beam: (1) a permanently deformed zone at the bottom; (2) an intermediate zone, which is not deformed at its bottom part and is deformed in the top part; and (3) a non-deformed zone at the top of the beam. They found that the occurrence and the development of these zones depends upon the dimensionless excitation amplitudes and the dimensionless frequency of the incident strong motion pulses, and in particular on the conditions that lead to the occurrence of the first permanent strain.

Gicev and Trifunac [2] have also showed that for excitation by near-field displacement pulses, failure can occur anywhere in the building, before the incident wave has completed its first travel from the foundation to the top of the building and back to the foundation.

For large and long strong-motion pulses, only zones 1 and 3 are present in the beam. For large amplitudes and short strong-motion pulses, all three zones can develop and are present. For smaller excitation amplitudes only zones 2 and 3 exist in the beam.

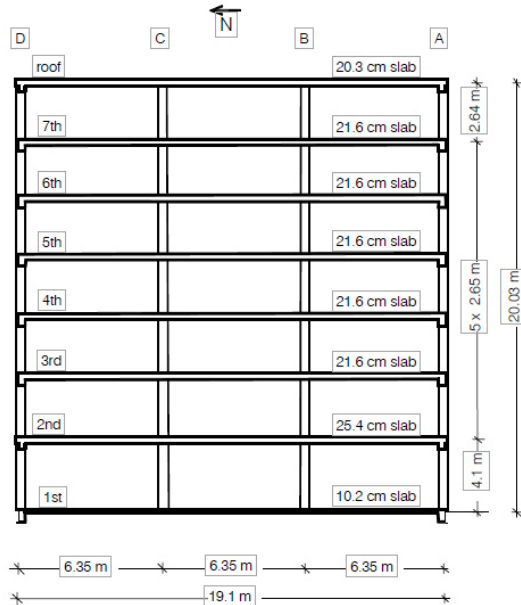


Figure 1. North – South section of the building.

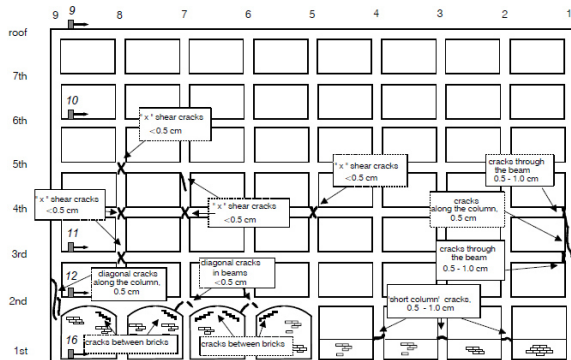


Figure 2. Observed damage at the north view of the building.



II. THE PHYSICS

A. Power Spectrum and Fourier Transform

In a study of the propagation of non-linear waves in a simple, uniform shear beam, caused by incident strong motion pulses, Gicev and Trifunac (2006) found that for large ground displacement pulses the maximum permanent strains in the beam occur mainly at the interface of the beam with the soil, while for smaller amplitudes of pulses permanent strains occur closer to the top of the beam. They identified three zones of the permanently deformed beam: (1) a permanently deformed zone at the bottom; (2) an intermediate zone, which is not deformed at its bottom part and is deformed in the top part; and (3) a non-deformed zone at the top of the beam. They found that the

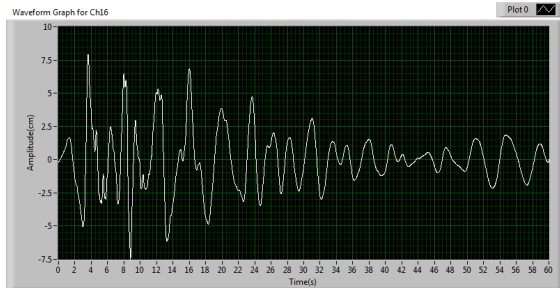


Figure 4. Displacement in matter of time for channel 16 (x-scale represent the time(s), y-scale represent the displacement (cm)).

If we analyse the waveform graph we can see that in first 24 seconds the amplitudes are high and are going from 5 to 7.5 centimeters.

From the channel 16 to channel 10 the amplitudes are continuously growing and at channel 10 they are highest. So because we are interested mostly of that what is the difference between the basement channel and the roof channel, on figure 5 is represented the displacement at channel 10.

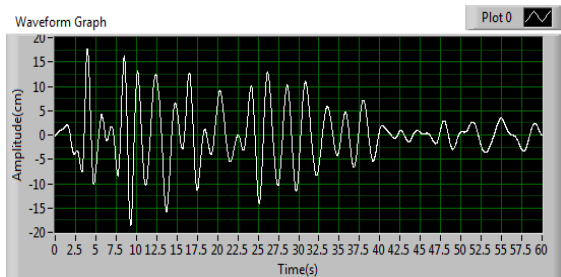


Figure 5. Displacement in matter of time for channel 10 (x-scale represent the time(s), y-scale represent the displacement (cm)).

From figure 5 can be seen that the displacement is significantly bigger and from 0 to 38 second is between nearly 8 to 20 centimeters, that gives us a clue that, metaforically said, the roof of the building is shaking for 20 centimeters. Important to mention is that these displacements looked from seismic angle are far more complex, and are illustrated in [2].

Next processing that interests us in this paper is the FFT of these signals.

The figure 8, waveform graph for channel 16 represent also FFT at displacements in the basement and prove that with smaller displacements there are smaller magnitudes, or, in case of ch16 we have 0.9 magnitude at frequency of 0.28Hz. The transfer

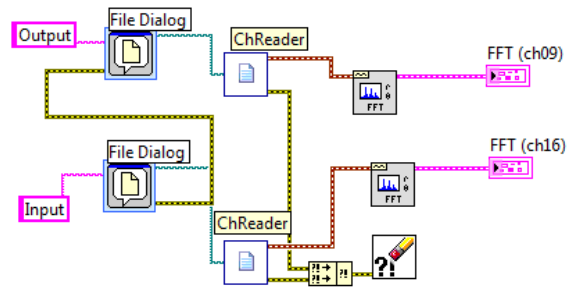


Figure 6. Blog diagram for Simultaneously reading the two channels with FFT signal processing

On figure 6 one can see that there are two channel readers connected with FFT spectrum for magnitudes of the signals. There are also two file-dialog boxes connected on the channels, and two waveform graphs denoted as FFT(ch10) and FFT(ch16).

The figure 7 and figure 8 represent the FFT of the channel 10 and channel 16 consequently. Now if we take a close look at waveform graph of channel 10 (crack point) we will see that for higher displacement we get higher magnitude. The highest peak is at nearly 0.44Hz at magnitude of 3.3.

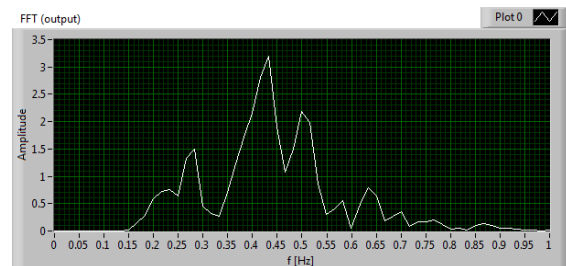


Figure 7. Waveform graph for FFT of channel 10 with x-scale frequency and y-scale magnitude.

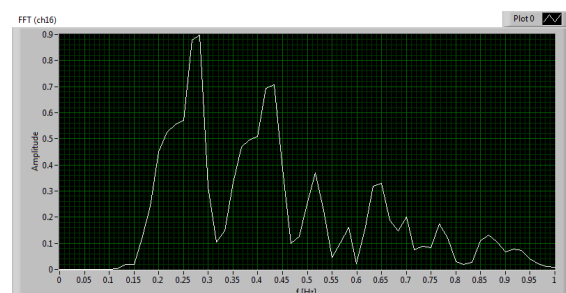


Figure 8. Waveform graph for FFT of channel 16 with x-scale frequency and y-scale magnitude.

function represents the ratio of the output of a system to the input to the system, in the Laplace domain considering its initial conditions and equilibrium point to be zero. If we have an input

function of $X(t)$, and an output function $Y(t)$, we define the transfer function $H(s)$ to be:

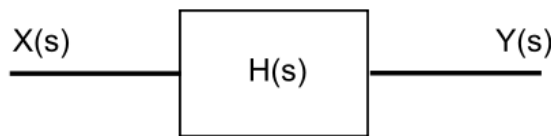
$$H(s) = \frac{Y(s)}{X(s)} \quad (2)$$


Figure 9. Input $X(s)$ and output $Y(s)$ of a transfer function $H(s)$.

The transfer function of a system is the relationship of the system's output to its input, represented in the complex Laplace domain.

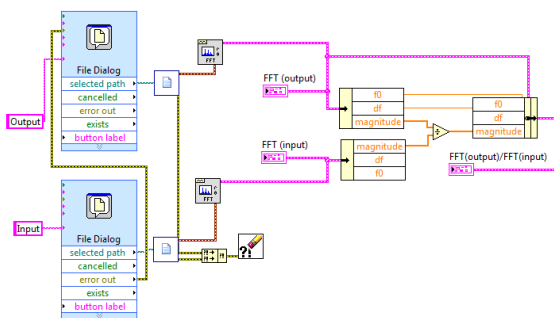


Figure 10. Block diagram of $H(t)$ transfer function via Labview

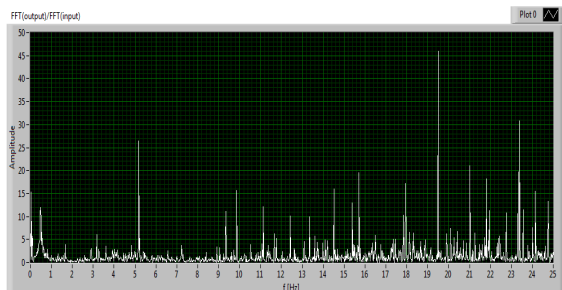


Figure 11. Waveform graph for $H(t)$ transfer function

Figure 10 represent the block diagram for $H(t)$ transfer function, the two channel readers gives the signals to FFT(magnitude) and we divide the output from the input in a waveform graph, figure 11. From the waveform graph we can see what's like the impulse response of this particular building or the natural frequency of the building. The highest amplitude peak is at nearly 4.5Hz with is crucial in case of a seismic excitation with that proportions.

IV. CONCLUSION

For past three of four decades, scientists are giving efforts on predicting some natural disasters. The fact is that, we still don't know all of the characteristics of these natural excitations, but we can prepare better for them, if we know closely the effects of these disasters in matter of their power. In our case we speak about seismic excitation that has result with damage to the VN7SH.

With power of numerical methods, mathematical transformations and software digital processings, we can be a little more aware of what kind of damage would be, in scenario like in California 1994. Every building has its own natural frequency. Sometimes it is very important to know this frequency because some buildings are used as fabriques that have some oscilating mashines and it can be of cruical importance where in the building will these mashines will be located. Our purpose is to take measurements of the buildings and tell to their builders little more informations of that how will that building act in worst-case scenario. Normally this field of investigation is improving more and more, with powerful tools like Labview which is used for these signal processing.

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